

City of Coquitlam
500 Mariner Way
Coquitlam, BC
V3B 7N2

December 13, 2023
File: 23549
R0

Attention: Pierce Redon

**Re: Geotechnical Investigation Report – Miller Park Field Upgrades
900 Oakview Street, Coquitlam, BC**

1.0 INTRODUCTION

We understand that it is proposed to upgrade the existing baseball field at the above referenced address. Based on information provided to us, we understand the upgrades would include a new fence and backstop, field upgrades, temporary grand stands supported on a concrete pad, and a new drainage system. We expect any new structures would be constructed at-grade and would exhibit very low loading on the underlying soils.

This report presents the results of our field investigation and provides recommendations for the design and construction of the proposed improvements. This report has been prepared exclusively for our client and members of their design team, and may be relied on by the City of Coquitlam during permitting and construction. No other use of this report is permitted without the written consent of GeoPacific.

2.0 SITE DESCRIPTION

The site is located at 900 Oakview Street in Coquitlam, BC. The site is approximately rectangular in shape, measuring 4.9 hectares in area. The baseball field is located at the southwest corner of the property. The baseball field is bounded by a forested area to the north, walking paths and a clearing to the east, residential developments to the south, and an existing school to the west. At the time of our investigation, the site was improved with the existing Miller Park baseball field. The existing field is essentially flat. The forested area to the north slopes down from south to north with a gradient of approximately 3H:1V, according to QtheMap.

The site location relative to the surrounding area is shown on our Drawing No. 23549-01 following the text of this report.

3.0 FIELD INVESTIGATION

3.1 Test Pit Investigation

The subsurface ground conditions of the above referenced site were investigated on November 29, 2023, using a track mounted excavator provided by D.W. Mini Excavating and Landscape of Langley, BC. At that time, a total of 8 test pits were excavated across the site and extended to depths up to 2 m below current site grade. Within 3 of the test pits, infiltration tests were conducted in order to determine the rate of infiltration of the native soils.

Prior to any excavating, a utility locate was completed by Municon West Coast to scan and clear any underground utilities. The test pits were logged in the field by a member of our geotechnical staff and backfilled in accordance with Provincial requirements immediately upon completion of logging and sampling or at the completion of the infiltration testing.

The locations of the test pits and infiltration tests are shown on our Drawing No. 23549-01 following the text of this report.

3.2 Infiltration Testing

Double Ring Infiltration Tests (DRITs) were completed in 3 of the test pits across the site. The DRITs were completed using a double ring infiltrometer consisting of 2 stainless steel rings measuring 300 mm and 600 mm in diameter. The premise of using 2 rings is to replicate one dimensional, vertical flow. The water within the annular area will flow vertically and laterally, creating a vertical flow path for the water within the inner ring.

The two rings were driven into the existing soils approximately 100 mm. The smaller ring was placed within the larger ring to form a central and annular area. The rings were filled with water and mini-piezometers were placed within the rings to record the pressure from the water head. An additional piezometer was placed outside of the rings to record atmospheric pressure changes. The piezometers record the water pressure within the annular space in 2 second intervals. The readings from the piezometer are normalized by the atmospheric pressure and plotted versus time. The slope of the graph indicates the infiltration rate.

4.0 RESULTS

4.1 Subsurface Soils

According to “Surficial Geology – New Westminster (Map 1484A)” published by the Geological Survey of Canada the area surrounding the subject site is understood to be underlain by Vashon Drift and Capilano Sediments consisting of lodgment and minor flow till, lenses and interbeds of substratified glaciofluvial sand to gravel, and lenses and interbeds of glaciolacustrine laminated stony silt. The site is located near the geological boundary of Capilano Sediments comprised of raised beach medium to coarse sand.

The soil profile consists of a thin layer of topsoil and grass, overlying silty sand or fill consisting of silty sand to sandy silt, overlying silt, then medium grained sand. A detailed description of the soils encountered is given below.

TOPSOIL

The site is surfaced in up to 0.3 m of topsoil and grass.

FILL

The topsoil is underlain by fill consisting of silty sand to sandy silt at the locations of TP23-02, TP23-03, TH23-04, and TP23-08. The fill was noted to extend up to 1.0 m below existing site grades. The fill is compact to firm, moist to wet, and brown in colour. Some roots and wood debris was observed within the fill strata. We expect the fill may be naturally deposited soils used to re-grade the original field. The moisture content of the fill is between 24.8 and 48.6% based on laboratory testing. The fill was not observed at the other test pit locations.

SILTY SAND

The topsoil is underlain by silty sand at the locations of TP23-05, TP23-06 and TP23-07 extending up to 0.5 m below existing site grades. The silty sand is compact, moist, and brown in colour. The silty sand was not observed at the other test pit locations.

SILT

The topsoil, fill or silty sand is underlain by silt at all the test pit locations extending up to 1.9 m below existing site grades. The silt is stiff, moist, and tan to grey in colour. The moisture content of the silt is between 23 and 27% based on laboratory testing.

SAND

The silt is underlain by medium grained sand at the location of TP23-01 extending beyond the depth of the investigation. The sand is compact, moist, and grey in colour.

4.2 Groundwater Conditions

The static groundwater table was not encountered during our investigation. Groundwater levels are expected to vary seasonally with higher levels in the wetter months of the year. Perched groundwater is expected to develop overlying the less permeable soils.

5.0 DISCUSSION

5.1 General Comments

As noted above, we understand the upgrades would include a new fence and backstop, field upgrades, temporary grand stands supported on a concrete pad, and a new drainage system. We expect any new structures would be constructed at-grade and would exhibit very low loading on the underlying soils.

We expect that any structures can be supported on conventional strip and pad footings. We anticipate that the footings will be founded on the naturally deposited compacted sand to silty sand, stiff silt, or engineered fill, which we expect will provide adequate bearing support.

We confirm, from a geotechnical point of view, that the proposed improvements are feasible provided that the recommendations outlined in the following sections are incorporated into the overall design and construction. GeoPacific should review any additional design details once selected and available.

6.0 DESIGN RECOMMENDATIONS

6.1 Site Preparation

Prior to the construction of any foundations or new field improvements, all organic materials, topsoil, loose or otherwise disturbed soil must be removed from the construction area to expose a subgrade consisting of naturally deposited compacted sand to silty sand, stiff silt. Based on our test pit investigation, we expect stripping depths to be up to 1.0 m. Stripping should extend out horizontally equal to the thickness of engineered fill being placed.

Any grade reinstatement should employ “engineered fill”. In the context of this report, engineered fill is defined as clean sand to sand and gravel, containing less than 5% fines by weight, compacted in 300 mm loose lifts to a minimum of 95% of the Modified Proctor (ASTM D1557) maximum dry density with a moisture content within 2% of optimum for compaction.

GeoPacific must be contacted to confirm the soil conditions during initial excavations for the proposed structure and confirm the stripping depths and compaction of engineered fill during construction.

6.2 Foundations and Bearing Capacity

Footings which are founded on the existing compact sand to silty sand or stiff silt, or engineered fill, as described in Section 4.1 above can be designed for a Serviceability Limit States (SLS) bearing pressure of 100 kPa. Factored ultimate limit state (ULS) bearing pressures, for transient loads such as those induced by wind and earthquakes, may be taken as 1.5 x the SLS bearing pressures provided above.

Irrespective of the allowable bearing pressures given, pad footings should not be less than 600 mm by 600 mm and strip footings should not be less than 450 mm in width. Foundations should also be buried a minimum of 450 mm below the surface for frost protection.

Post construction settlement for lightly loaded structures designed as recommended should be less than 25 mm total 1:500 differential.

Foundation subgrades must be reviewed by GeoPacific prior to footing construction.

6.3 Seismic Design of Foundations

The site qualifies as “Site Class C” as defined in Table 4.1.8.4.A of the 2018 BCBC. Based on our geotechnical investigation and previously undertaken investigations in the area, the subsurface soils are not considered prone to ground liquefaction or other forms of ground softening caused by earthquake induced ground motions. We have considered an earthquake having a peak horizontal ground acceleration of 0.33 g for this site (Ref. Natural Resources Canada, 2015, Site Coordinates: 49.2652 N and 122.8726 W).

6.4 Concrete Slab Preparation

As noted, we understand new concrete pads will be constructed to support temporary grandstands. After completion of the recommended site preparation, concrete slabs should be directly underlain by a minimum of 150 mm of free draining granular material, such as 19 mm clear crush gravel, compacted sufficiently to an equivalent of 95% Modified Proctor Dry Density (ASTM D1557) at a moisture content that is within 2% of optimum for compaction. General grade reinstatement or backfill beneath slab-on-grade areas should be done with engineered fill, as described in Section 6.1.

All slab-on-grade preparation must be reviewed by the geotechnical engineer prior to slab-on-grade construction.

6.5 Storm Water Management

A total of 3 infiltration tests were completed at the site to aid in the civil design of storm water management infrastructure that may rely on natural infiltration. Tests were completed at depths of between 1.2 m and 1.5 m below current site grades in sand or silt strata. Unfactored infiltration rates are presented in Table 2 below. Detailed results are presented in Appendix B following the text of this report.

Table 1: Unfactored Infiltration Rates

Location	Test Depth (m)	Infiltration Rate (mm/hr)
DRIT23-01	1.2	1550
DRIT23-02	1.5	25
DRIT23-03	1.2	27

The results of the testing indicate the medium grained has a high infiltration potential, however, the silt has a relatively low infiltration potential. Any stormwater management systems should consider an unfactored infiltration rate of 25 mm/hr.

If higher rates are required, GeoPacific can complete further testing in the area of TP23-01 to better delineate the sand strata.

All subgrade soil beneath stormwater management systems that rely on natural infiltration should be reviewed by the geotechnical engineer prior to construction.

6.6 Temporary Excavations and Utility Installation

We expect some shallow excavations may be required for utility installation. Excavations in excess of 1.2 metres in height, requiring worker-entry, require review by a Professional Engineer in accordance with WorkSafe BC.

Seepage from the surficial soils and perched groundwater is expected to be relatively light. We expect that excavation inflows may be handled with conventional sumps and sump pumps.

We recommend that all trench fill and bedding materials conform to MMCD and be compacted to a minimum of 95% MPDD.

Excavations deeper than 1.2 m must be reviewed by a Professional Engineer prior to worker entry.

7.0 DESIGN REVIEWS AND CONSTRUCTION REVIEWS

As required for Municipal “Letters of Assurance”, GeoPacific Consultants Ltd. will carry out sufficient field reviews during construction to ensure that the geotechnical design recommendations contained within this report have been adequately communicated to the design team and to the contractors implementing the design. These field reviews are not carried out for the benefit of the contractors and therefore do not in any way effect the contractors’ obligations to perform under the terms of his/her contract.

It is the contractors’ responsibility to advise GeoPacific Consultants Ltd. (a minimum of 48 hours in advance) that a field review is required. Field reviews are normally required at the time of the following activities:

- | | |
|--------------------|--|
| 1. Stripping | Review of stripping depth and subgrade. |
| 2. Subgrade | Review of subgrade soils prior to foundation construction. |
| 3. Excavation | Review of slope cuts and excavations greater than 1.2 m deep requiring worker entry. |
| 4. Engineered Fill | Review of materials and compaction degree. |

It is critical that these reviews are carried out to ensure that our intentions have been adequately communicated. It is also critical that contractors working on the site view this document in advance of any work being carried out so that they become familiar with the sensitive aspects of the works proposed. It is the responsibility of the developer to notify GeoPacific Consultants Ltd. when conditions or situations not outlined within this document are encountered.

8.0 CLOSURE

This report has been prepared exclusively for our client for the purpose of providing geotechnical recommendations for the design and construction of the proposed improvements. The report remains the property of GeoPacific Consultants Ltd. and unauthorized use of, or duplication of this report is prohibited.

We are pleased to be of assistance to you on this project and we trust that our comments and recommendations are both helpful and sufficient for your current purposes. If you would like further details or would like clarification of any of the above, please do not hesitate to call.

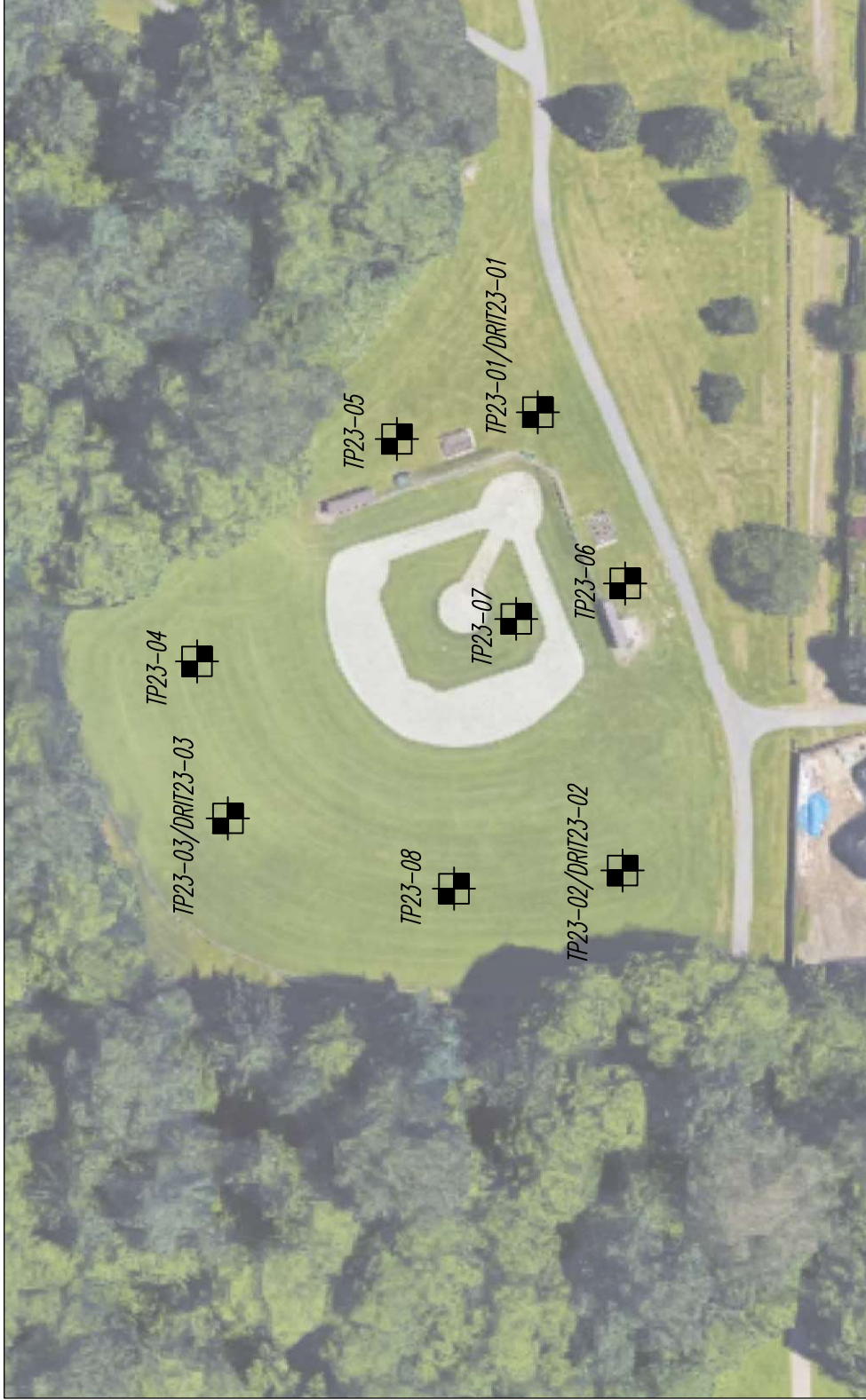
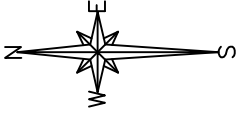
For:

GeoPacific Consultants Ltd.

Reviewed By:

Austin Lockstidt, B.A.Sc., EIT
Engineer In Training

Alex Gossen, M.Eng., P.Eng.
Geotechnical Engineer



LEGEND:

TP23-00  -- APPROXIMATE TEST PIT (TP) LOCATIONS



GEOPACIFIC
VANCOUVER KALISLOUS CALABAY

1770 W. 55th Avenue
Vancouver, B.C. V6P 4P9
P 604 433-6622
F 604 439-8589

DATE: NOVEMBER 27, 2023			
DRAWN BY: ALO	APPROVED BY: AG	REVIEWED BY: AG	
SCALE:		NOT TO SCALE	

MILLER PARK FIELD UPGRADES
900 OAKVIEW STREET, COQUITLAM, BC
APPROXIMATE GEOTECHNICAL INVESTIGATION AREA

FILE NO.: 23549	REVISIONS:
DWG. NO.: 23549-01	A.
	B.
	C.

REFERENCE:

APPENDIX A – TEST PIT LOGS

Test Pit Log: TP23-01/DRIT23-01

File: 23549

Project: Miller Park Field Upgrades

Client: City of Coquitlam

Site Location: 900 Oakview Street, Coquitlam, BC



GEOPACIFIC
CONSULTANTS

1779 West 75th Avenue, Vancouver, BC, V6P 6P2
Tel: 604-439-0922 Fax: 604-439-9189

INFERRED PROFILE				Moisture Content (%)	Groundwater	Remarks
Depth	Symbol	SOIL DESCRIPTION	Depth/Elev (m)			
0		Ground Surface				
0		Topsoil	0.0			
1		Silt Silt, trace sand, stiff, moist, tan to grey.	0.2			
2				21.3		
3		Sand Medium grained sand, compact, moist, grey.	0.8			
4						
5						
6						
7		End of Test Pit	2.0			
8						
9						
3						

Infiltration rate of 1546.9 mm/hr at 1.2 m.

Logged: ALO

Method: Excavator

Date: November 29, 2023

Datum: Ground Surface

Figure Number: A.01

Page: 1 of 1

Test Pit Log: TP23-02/DRIT23-02

File: 23549

Project: Miller Park Field Upgrades

Client: City of Coquitlam

Site Location: 900 Oakview Street, Coquitlam, BC



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Tel: 604-439-0922 Fax: 604-439-9189

INFERRED PROFILE				Moisture Content (%)	Groundwater	Remarks
Depth	Symbol	SOIL DESCRIPTION	Depth/Elev (m)			
0		Ground Surface				
0		Topsoil	0.0			
1		Sandy Silt (Fill) Sandy silt fill, firm, wet, brown. Trace roots and wood debris.	0.2			
2						
3				47.4		FC: 55.4% at 0.9 m.
4						
5		Silt Silt, trace sand, stiff, moist, tan to grey.	1.3			Infiltration rate of 24.9 mm/hr at 1.5 m.
6						
7		End of Test Pit	1.9			
8						
9						
3						

Logged: ALO

Method: Excavator

Date: November 29, 2023

Datum: Ground Surface

Figure Number: A.02

Page: 1 of 1

Test Pit Log: TP23-03/DRIT23-03

File: 23549

Project: Miller Park Field Upgrades

Client: City of Coquitlam

Site Location: 900 Oakview Street, Coquitlam, BC



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Tel: 604-439-0922 Fax: 604-439-9189

INFERRED PROFILE				Moisture Content (%)	Groundwater	Remarks
Depth	Symbol	SOIL DESCRIPTION	Depth/Elev (m)			
0		Ground Surface				
0		Topsoil	0.0			
1		Silty Sand (Fill) Silty sand fill, compact, moist, brown. Trace roots and wood debris.	0.2			
2						
3		Silt Silt, trace sand, stiff, moist, tan to grey.	0.9	23.0		
4						
5		End of Test Pit	1.4			Infiltration rate of 27.0 mm/hr at 1.2 m.
6						

Logged: ALO

Method: Excavator

Date: November 29, 2023

Datum: Ground Surface

Figure Number: A.03

Page: 1 of 1

Test Pit Log: TP23-04

File: 23549

Project: Miller Park Field Upgrades

Client: City of Coquitlam

Site Location: 900 Oakview Street, Coquitlam, BC



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INFERRED PROFILE				Moisture Content (%)	Groundwater	Remarks
Depth	Symbol	SOIL DESCRIPTION	Depth/Elev (m)			
0		Ground Surface				
0		Topsoil	0.0			
1		Sandy Silt (Fill) Sandy silt fill, firm, wet, brown. Trace roots and wood debris.				
2						
3		Silt Silt, trace sand, stiff, moist, tan to grey.	0.9	48.6		
4						
5				25.3		FC: 90.2% at 1.3 m.
6		End of Test Pit	1.6			

Logged: ALO

Method: Excavator

Date: November 29, 2023

Datum: Ground Surface

Figure Number: A.04

Page: 1 of 1

Test Pit Log: TP23-05

File: 23549

Project: Miller Park Field Upgrades

Client: City of Coquitlam

Site Location: 900 Oakview Street, Coquitlam, BC



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Tel: 604-439-0922 Fax: 604-439-9189

INFERRED PROFILE				Moisture Content (%)	Groundwater	Remarks
Depth	Symbol	SOIL DESCRIPTION	Depth/Elev (m)			
0 ft 0 m		Ground Surface				
		Topsoil	0.0			
1		Silty Sand Silty sand, compact, moist, brown.	0.3			
2		Silt Silt, trace sand, stiff, moist, tan to grey.	0.5			
3						
4				24.5		
5						
6		End of Test Pit	1.8			

Logged: ALO

Method: Excavator

Date: November 29, 2023

Datum: Ground Surface

Figure Number: A.05

Page: 1 of 1

Test Pit Log: TP23-06

File: 23549

Project: Miller Park Field Upgrades

Client: City of Coquitlam

Site Location: 900 Oakview Street, Coquitlam, BC



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1779 West 75th Avenue, Vancouver, BC, V6P 6P2
Tel: 604-439-0922 Fax: 604-439-9189

INFERRED PROFILE				Moisture Content (%)	Groundwater	Remarks
Depth	Symbol	SOIL DESCRIPTION	Depth/Elev (m)			
0		Ground Surface				
0		Topsoil	0.0			
1		Silty Sand Silty sand, compact, moist, brown.	0.3			
2		Silt Silt, trace sand, stiff, moist, tan to grey.	0.5			
3				27.0		
4						
5		End of Test Pit	1.5			
6						

Logged: ALO

Method: Excavator

Date: November 29, 2023

Datum: Ground Surface

Figure Number: A.06

Page: 1 of 1

Test Pit Log: TP23-07

File: 23549

Project: Miller Park Field Upgrades

Client: City of Coquitlam

Site Location: 900 Oakview Street, Coquitlam, BC



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1779 West 75th Avenue, Vancouver, BC, V6P 6P2
Tel: 604-439-0922 Fax: 604-439-9189

INFERRED PROFILE				Moisture Content (%)	Groundwater	Remarks
Depth	Symbol	SOIL DESCRIPTION	Depth/Elev (m)			
0 ft 0 m		Ground Surface				
		Topsoil	0.0			
		Silty Sand Silty sand, compact, moist, brown.	0.2			
		Silt Silt, trace sand, stiff, moist, tan to grey.	0.4			
				26.1		
1 ft 1 m		End of Test Pit	1.0			
2 ft 2 m						
3 ft 3 m						
4 ft 4 m						

Logged: ALO

Method: Excavator

Date: November 29, 2023

Datum: Ground Surface

Figure Number: A.07

Page: 1 of 1

Test Pit Log: TP23-08

File: 23549

Project: Miller Park Field Upgrades

Client: City of Coquitlam

Site Location: 900 Oakview Street, Coquitlam, BC



GEOPACIFIC
CONSULTANTS

1779 West 75th Avenue, Vancouver, BC, V6P 6P2
Tel: 604-439-0922 Fax: 604-439-9189

INFERRED PROFILE				Moisture Content (%)	Groundwater	Remarks
Depth	Symbol	SOIL DESCRIPTION	Depth/Elev (m)			
0		Ground Surface				
0		Topsoil	0.0			
1		Silty Sand (Fill) Silty sand fill, compact, moist, brown. Trace roots and wood debris.	0.2			
2						
3				24.8		
4		Silt Silt, trace sand, stiff, moist, tan to grey.	1.0			
5		End of Test Pit	1.5			
6						

Logged: ALO

Method: Excavator

Date: November 29, 2023

Datum: Ground Surface

Figure Number: A.08

Page: 1 of 1

APPENDIX B – INFILTRATION TEST RESULTS



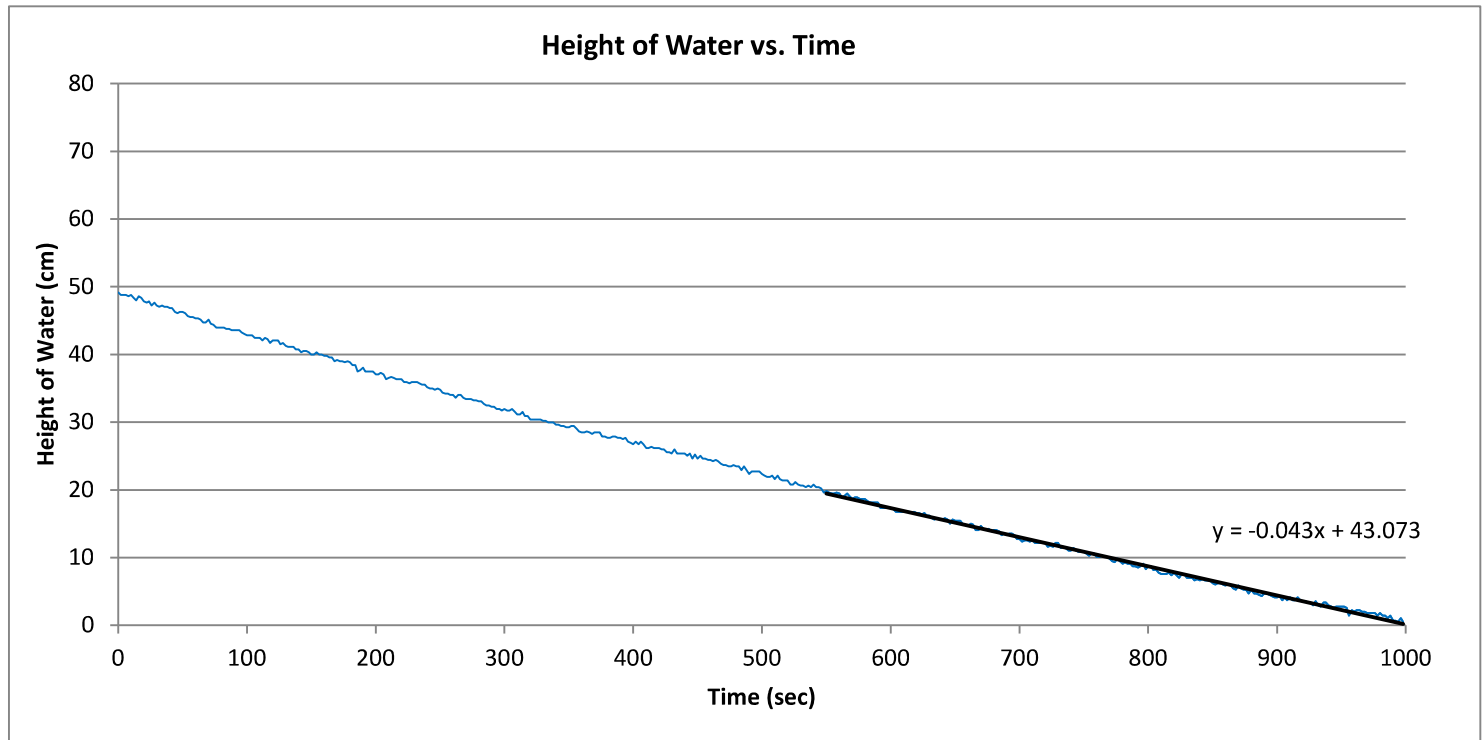
GEOPACIFIC
VANCOUVER KAMLOOPS CALGARY

**DOUBLE RING
INFILTRATION TEST
(ASTM D3385)**

Vancouver Lab
1779 West 75th Avenue
Vancouver, B.C
V6P 6P2

CLIENT:	CITY OF COQUITLAM	PROJECT #:	23549
PROJECT NAME:	MILLER PARK FIELD UPGRADES	DATE:	29-Nov-2023 9:00
PROJECT LOCATION:	900 OAKVIEW STREET, COQUITLAM	TEST NO.:	DRIT23-01

ESTIMATED FIELD SATURATED INFILTRATION RATE: 1546.9 mm/Hr



TEST INFORMATION					
LIQUID USED:	WATER	AREA INNER CYLINDER:	72965.63 mm ²	DEPTH OF TEST BELOW CURRENT SITE GRADE (m):	1.2
LIQUID pH:	7.1	ANNULAR AREA:	209777.71 mm ²		
SOIL DESCRIPTION:	SAND	CYLINDER ACCURACY:	± 0.5 mm	INFILTRATION RATE (cm/sec):	4.297E-02
SOIL TYPE:	NATIVE	LIQUID DEPTH INNER:	49.2 cm		
WEATHER ON SITE:	CLEAR	LIQUID DEPTH OUTER:	N/A	DEPTH TO WATER TABLE:	UNKNOWN

Comments:

Per: Austin Lockstidt, B.A.Sc., EIT

Engineer In Training

Reviewed by: Jakub Szary, B.Sc., ASCT

Lab Manager



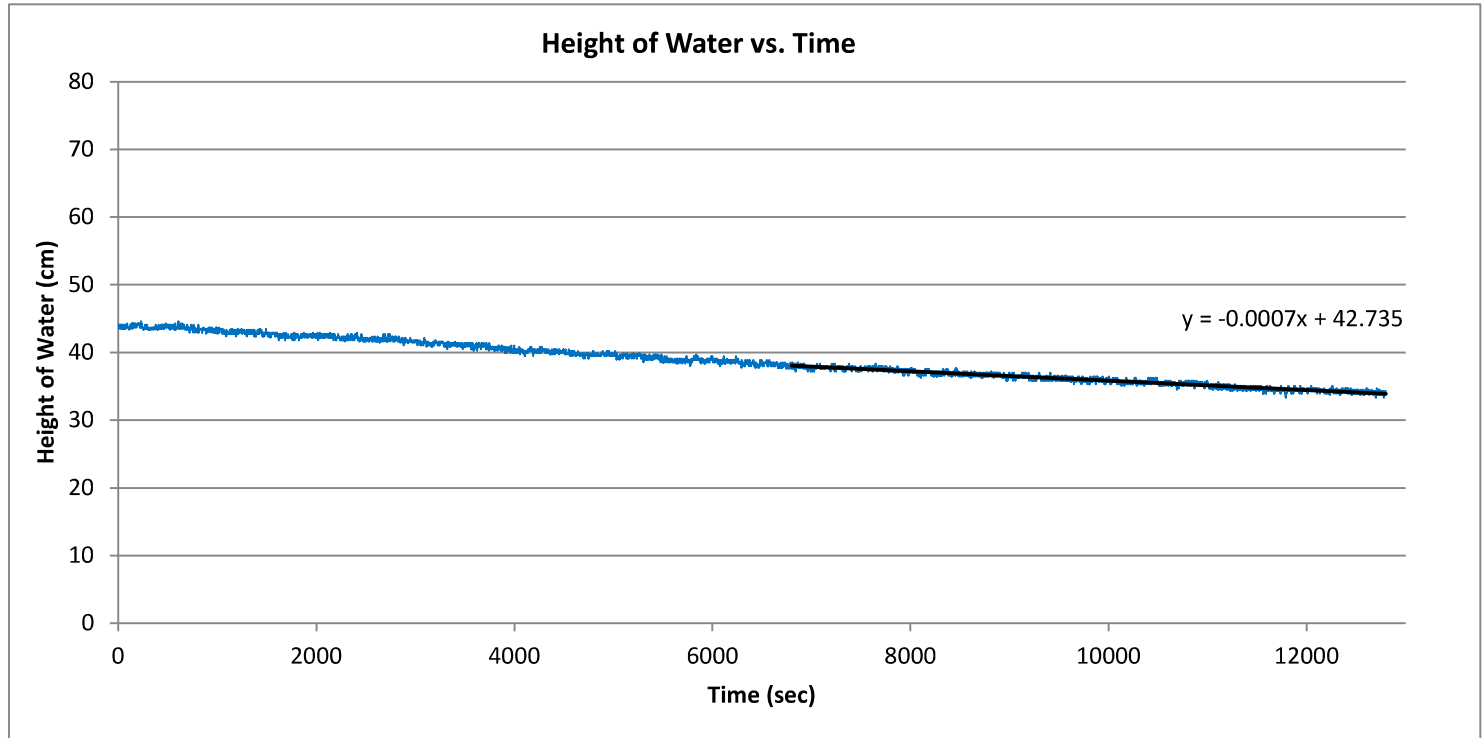
GEOPACIFIC
VANCOUVER KAMLOOPS CALGARY

**DOUBLE RING
INFILTRATION TEST
(ASTM D3385)**

Vancouver Lab
1779 West 75th Avenue
Vancouver, B.C
V6P 6P2

CLIENT:	CITY OF COQUITLAM	PROJECT #:	23549
PROJECT NAME:	MILLER PARK FIELD UPGRADES	DATE:	29-Nov-2023 9:15
PROJECT LOCATION:	900 OAKVIEW STREET, COQUITLAM	TEST NO.:	DRIT23-02

ESTIMATED FIELD SATURATED INFILTRATION RATE: 24.9 mm/Hr



TEST INFORMATION					
LIQUID USED:	WATER	AREA INNER CYLINDER:	72965.63 mm ²	DEPTH OF TEST BELOW CURRENT SITE GRADE (m):	1.5
LIQUID pH:	7.1	ANNULAR AREA:	209777.71 mm ²		
SOIL DESCRIPTION:	SILT, TRACE SAND	CYLINDER ACCURACY:	± 0.5 mm	INFILTRATION RATE (cm/sec):	6.912E-04
SOIL TYPE:	NATIVE	LIQUID DEPTH INNER:	44.7 cm		
WEATHER ON SITE:	CLEAR	LIQUID DEPTH OUTER:	N/A	DEPTH TO WATER TABLE:	UNKNOWN

Comments:

Per: Austin Lockstidt, B.A.Sc., EIT

Engineer In Training

Reviewed by: Jakub Szary, B.Sc., ASCT

Lab Manager



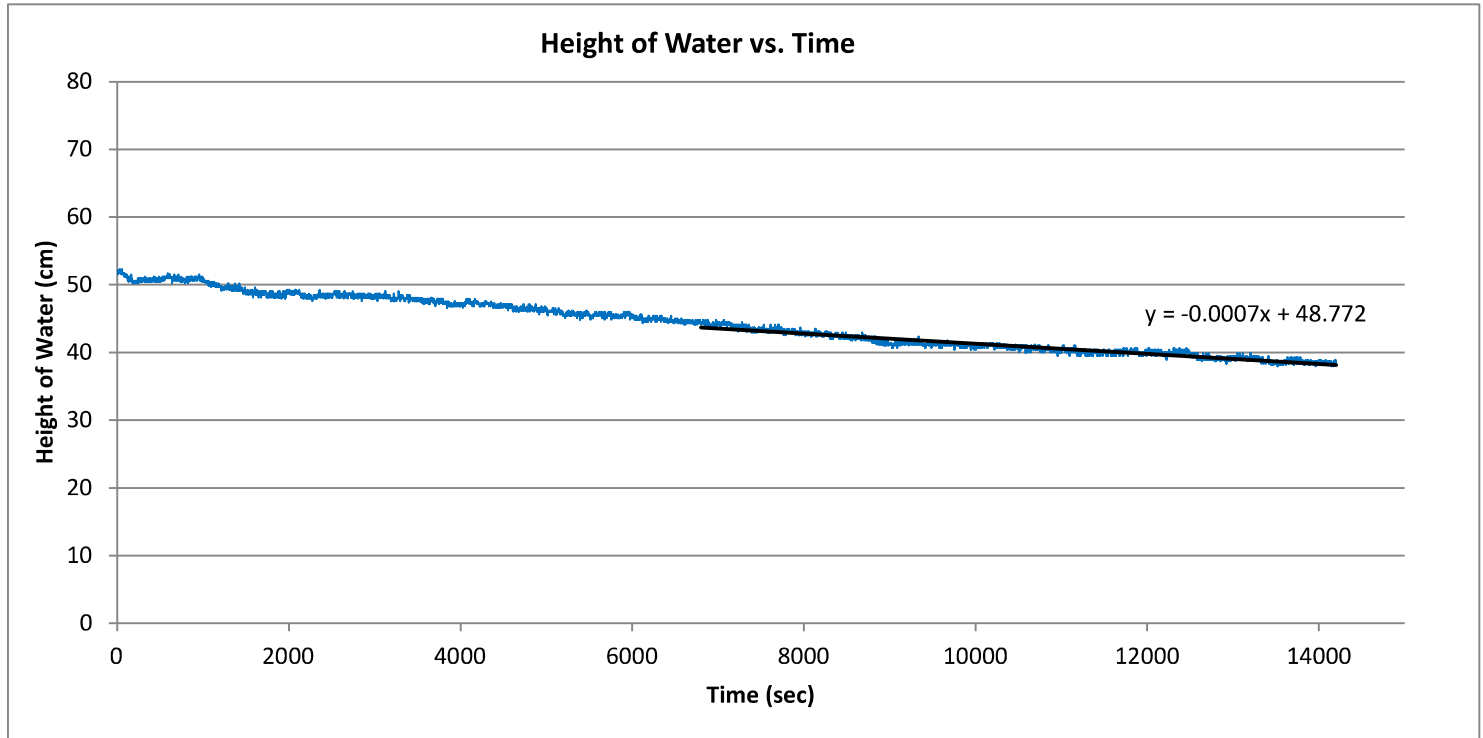
GEOPACIFIC
VANCOUVER KAMLOOPS CALGARY

DOUBLE RING
INFILTRATION TEST
(ASTM D3385)

Vancouver Lab
1779 West 75th Avenue
Vancouver, B.C
V6P 6P2

CLIENT:	CITY OF COQUITLAM	PROJECT #:	23549
PROJECT NAME:	MILLER PARK FIELD UPGRADES	DATE:	29-Nov-2023 9:30
PROJECT LOCATION:	900 OAKVIEW STREET, COQUITLAM	TEST NO.:	DRIT23-03

ESTIMATED FIELD SATURATED INFILTRATION RATE: 27.0 mm/Hr



TEST INFORMATION					
LIQUID USED:	WATER	AREA INNER CYLINDER:	72965.63 mm ²	DEPTH OF TEST BELOW CURRENT SITE GRADE (m):	1.2
LIQUID pH:	7.1	ANNULAR AREA:	209777.71 mm ²		
SOIL DESCRIPTION:	SILT, TRACE SAND	CYLINDER ACCURACY:	± 0.5 mm	INFILTRATION RATE (cm/sec):	7.490E-04
SOIL TYPE:	NATIVE	LIQUID DEPTH INNER:	52.3 cm		
WEATHER ON SITE:	CLEAR	LIQUID DEPTH OUTER:	N/A	DEPTH TO WATER TABLE:	UNKNOWN

Comments:

Per: Austin Lockstidt, B.A.Sc., EIT

Engineer In Training

Reviewed by: Jakub Szary, B.Sc., ASCT

Lab Manager

APPENDIX C – LAB RESULTS

**FINES CONTENT REPORT
(ASTM C117)**

CLIENT:	CITY OF COQUITLAM	JOB #:	23549
PROJECT:	MILLER PARK FIELD AND BACKSTOP REPLACEMENT	RECEIVED:	29-Nov-23
LOCATION:	900 OAKVIEW STREET, COQUITLAM	TESTED:	1-Dec-23
		REPORT #:	1

HOLE #:	TP23 - 02	TP23 - 04			
DEPTH:	0.9 m	1.3 m			
FINES CONTENT:	55.4%	90.2%			

HOLE #:					
DEPTH:					
FINES CONTENT:					

HOLE #:					
DEPTH:					
FINES CONTENT:					

HOLE #:					
DEPTH:					
FINES CONTENT:					

HOLE #:					
DEPTH:					
FINES CONTENT:					

HOLE #:					
DEPTH:					
FINES CONTENT:					

COMMENTS:

DISTRIBUTION:

lockstidt@geopacific.ca

Per: Darragh Fitzgerald, B.Sc.

Reviewed By: Cindy Marinovic, B.Sc.

Cindy Marinovic

Lab Technician

Lab Supervisor





MOISTURE CONTENT
(ASTM D2216)

CLIENT:	CITY OF COQUITLAM	JOB #:	23549
PROJECT:	MILLER PARK FIELD AND BACKSTOP REPLACEMENT	RECEIVED:	29-Nov-23
LOCATION:	900 OAKVIEW STREET, COQUITLAM	TESTED:	30-Nov-23

REPORT #:	1
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HOLE #:	TP23 - 01	TP23 - 02	TP23 - 03	TP23 - 04	TP23 - 04
DEPTH:	0.5 m	0.9 m	1.0 m	0.8 m	1.3 m
M/C:	21.3%	47.4%	23.0%	48.6%	25.3%

HOLE #:	TP23 - 05	TP23 - 06	TP23 - 07	TP23 - 08	
DEPTH:	1.1 m	1 m	0.8 m	0.9 m	
M/C:	24.5%	27.0%	26.1%	24.8%	

HOLE #:					
DEPTH:					
M/C:					

HOLE #:					
DEPTH:					
M/C:					

HOLE #:					
DEPTH:					
M/C:					

HOLE #:					
DEPTH:					
M/C:					

COMMENTS:

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Lab Technician

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